

# ***SFHS Meeting Coral Springs, FL May 8th, 2013***

## **HERBERT HOOVER DIKE REHABILITATION “THE IMPORTANCE OF DAM SAFETY IN SOUTH FLORIDA”**

**Presented By:**

**Dan Schauer, P.G.**

**Principal Geologist, Geosyntec Consultants, Boca Raton, FL**

**Geosyntec**  
consultants

engineers | scientists | innovators



# PRESENTATION OUTLINE

- ❖ USACE Dam Safety Program
- ❖ HHD History
- ❖ Cutoff Wall Design
- ❖ CSM Method
- ❖ QA/QC
- ❖ Data Management

Geosyntec  
consultants

engineers | scientists | innovators



# USACE DAM SAFETY PROGRAM

- Risk & Reliability-based approach adopted since early 2000s to manage portfolio of 694 dams in US & PR
- **Public safety the number one priority**
- Flood damage to levees during the Katrina disaster gave new urgency to addressing the aging infrastructure Corps portfolio of dams and levees

# *USACE DAM SAFETY PROGRAM*

- Approximately 15 million people are at risk from USACE dams
- Program adopted to develop balanced and informed assessments
- Evaluate, prioritize and justify dam safety decisions

# *USACE DAM SAFETY PROGRAM*

- Approximately 95 percent of the dams managed by USACE are more than 30 years old
- 52 percent have reached or exceeded the 50-year service life
- \$25 billion a year in economic benefits
- The 2013 National Inventory of Dams database is now available!

# DAM SAFETY ACTION CLASSIFICATION



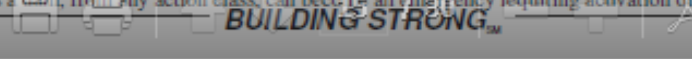
US Army Corps of Engineers



Table 3.1 USACE Dam Safety Action Classification Table\* 6 May 2008 version

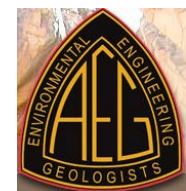
Dam Safety Action Class	Characteristics of this class	Actions for dams in this class
<b>I</b> <b>URGENT AND COMPELLING</b> (Unsafe)	<b>CRITICALLY NEAR FAILURE</b> Progression toward failure is confirmed to be taking place under normal operations. Almost certain to fail under normal operations from immediately to within a few years without intervention. <b>OR EXTREMELY HIGH RISK</b> Combination of life or economic consequences with probability of failure is extremely high.	Take immediate action to avoid failure. Validate classification through an external peer review. Implement interim risk reduction measures, including operational restrictions, and ensure that emergency action plan is current and functionally tested for initiating event Conduct heightened monitoring and evaluation. Expedite investigations to support justification for remediation using all resources and funding necessary. Initiate intensive management and situation reports.
<b>II</b> <b>URGENT</b> (Unsafe or Potentially Unsafe)	<b>FAILURE INITIATION FORESEEN</b> For confirmed (unsafe) and unconfirmed (potentially unsafe) dam safety issues, failure could begin during normal operations or be initiated as the consequence of an event. The likelihood of failure from one of these occurrences, prior to remediation, is too high to assure public safety. <b>OR VERY HIGH RISK</b> The combination of life or economic consequences with probability of failure is very high.	Implement interim risk reduction measures, including operational restrictions as justified, and ensure that emergency action plan is current, and functionally tested for initiating event Conduct heightened monitoring and evaluation. Expedite confirmation of classification. Give very high priority for investigations to support justification for remediation.
<b>III</b> <b>HIGH PRIORITY</b> (Conditionally Unsafe)	<b>SIGNIFICANTLY INADEQUATE OR MODERATE TO HIGH RISK</b> For confirmed and unconfirmed dam safety issues, the combination of life, economic, or environmental consequences with probability of failure is moderate to high.	Implement interim risk reduction measures, including operational restrictions as justified, and ensure that emergency action plan is current and functionally tested for initiating event Conduct heightened monitoring and evaluation. Prioritize for investigations to support justification for remediation considering consequences and other factors.
<b>IV</b> <b>PRIORITY</b> (Marginally Safe)	<b>INADEQUATE WITH LOW RISK</b> For confirmed and unconfirmed dam safety issues, the combination of life, economic, or environmental consequences with probability of failure is low and may not meet all essential USACE guidelines.	Conduct elevated monitoring and evaluation. Give normal priority to investigations to validate classification, but no plan for risk reduction measures at this time.
<b>V</b> <b>NORMAL</b> (Adequately Safe)	<b>ADEQUATELY SAFE</b> Dam is considered safe, meeting all essential USACE guidelines with no unconfirmed dam safety issues. <b>AND RESIDUAL RISK IS CONSIDERED TOLERABLE.</b>	Continue routine dam safety activities, normal operation, and maintenance.

\* At any time for specific events a dam, from any action class, can become an emergency requiring activation of the emergency plan



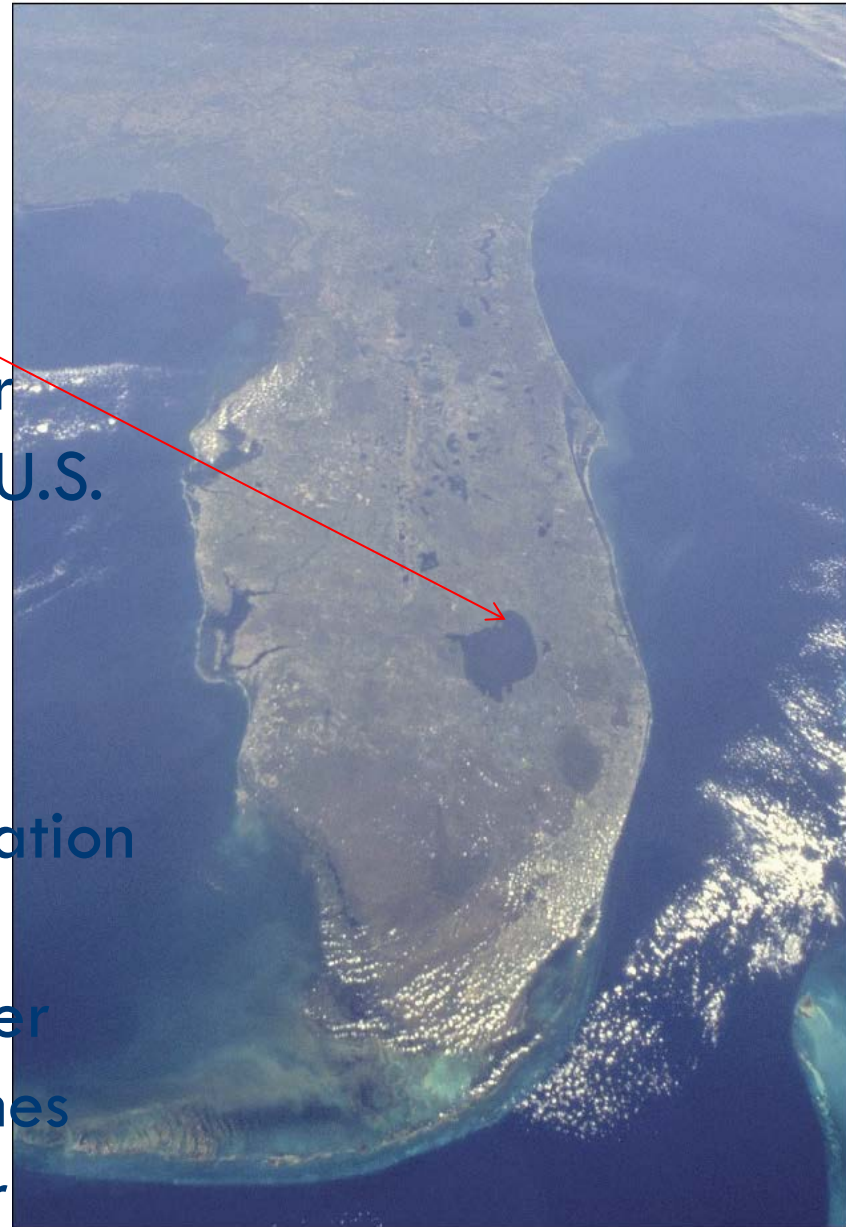
# USACE PARTNERS

- Institute for Water Resources
- Bureau of Reclamation
- Federal Energy Regulatory Commission
- ASDSO
- US Society of Dams
- Association of Engineering Geologists



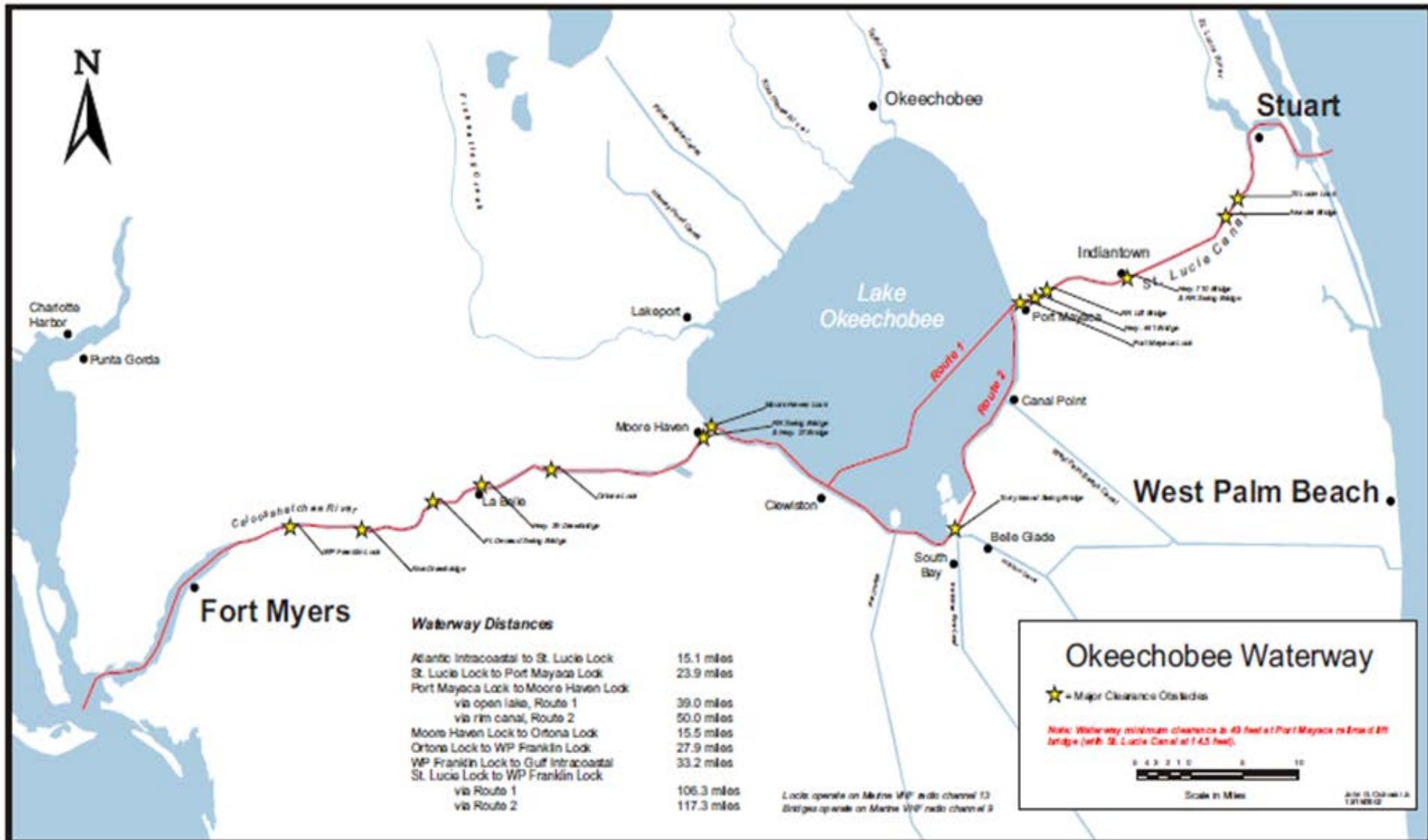
# Lake Okeechobee

- 730 mi<sup>2</sup>
- 2nd Largest Freshwater
- Lake w/in continental U.S.
- Depth ~12 feet
- 154 mile cross-state  
waterway
- Drinking Water & Irrigation
- Recreation
- 143-mile dike built after  
1926 & 1928 Hurricanes
- Changed natural water  
flow





# OKEECHOBEE WATERWAY



# History Of Dike Construction



*Great Miami Hurricane  
Great Okeechobee / San  
Felipe Hurricane\**

The River and Harbor Act of 1930 authorized construction of 67.8 miles of levee along the south shore and 15.7 miles of levee along the north shore of Lake Okeechobee.

1930's – 1960's



Levee Construction South Side  
Lake Okeechobee C. 1933



Levee Const. Near Rubbin Slough  
Lake Okeechobee C. 1935

# CONSTRUCTION FLAWS

- No selection of fill materials or use of engineered fill placement techniques
- No seepage control features through embankment or foundation
- Ditches on both sides of the dike exposed high porosity foundation
- Were no Dam Safety Standards in 1932



FIG. 2  
SERIOUS EROSION ON UNPROTECTED LEVEE FACE

NOTE:

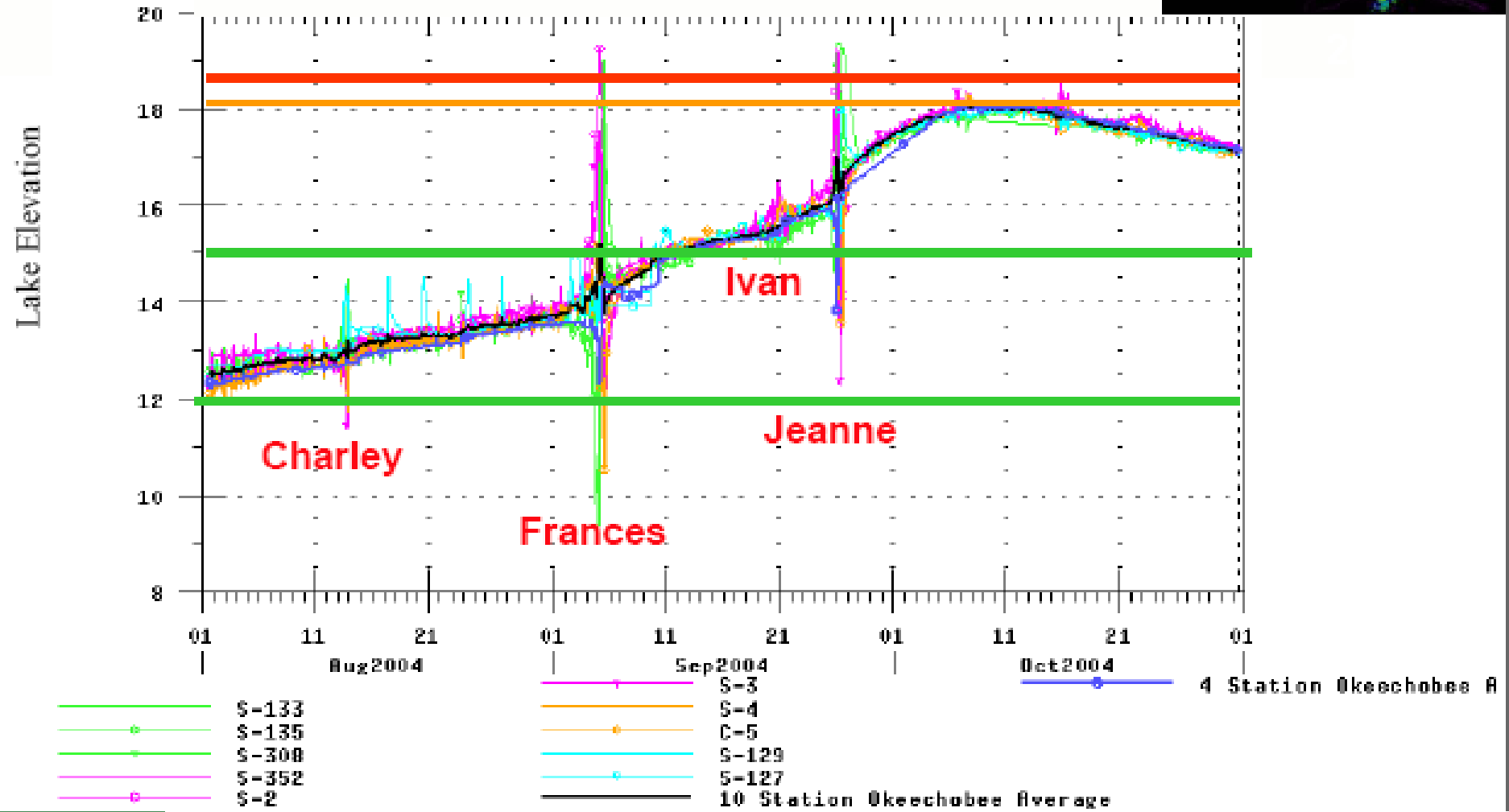
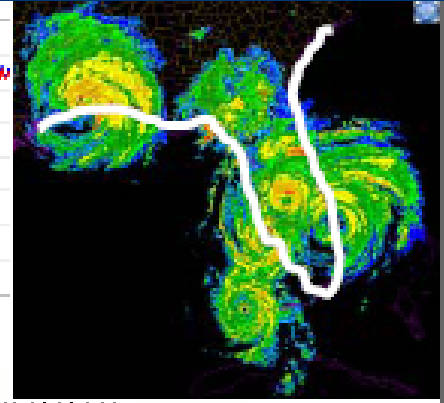
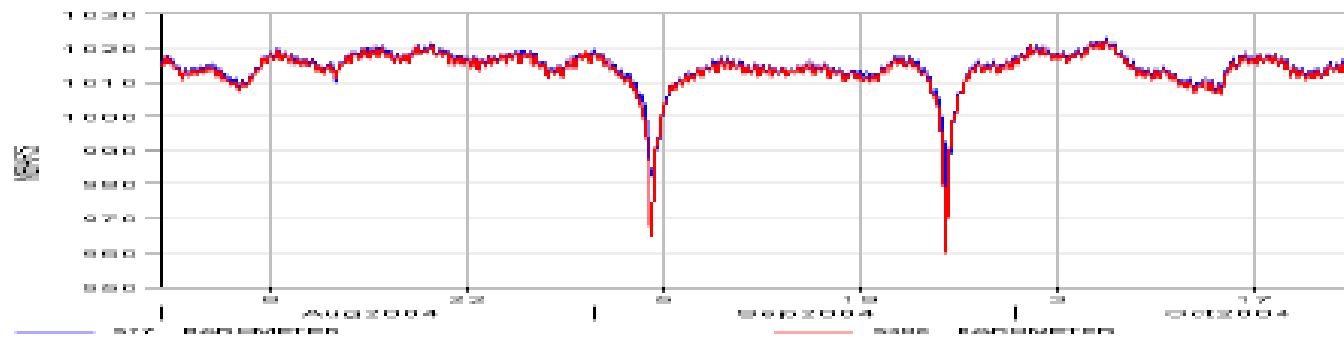
LEVEE EROSION ABOUT WIDWAY BETWEEN  
HURRICANE GATE STRUCTURES NOS. 2 AND  
3, CAUSED BY THE HURRICANE OF  
SEPTEMBER 17, 1947.

CENTRAL AND SOUTHERN FLORIDA  
LAKE OKEECHOBEE

LEVEE EROSION-1947 HURRICANE

JACKSONVILLE DISTRICT, CORPS OF ENGINEERS  
TO ACCOMPANY PARTIAL DPR, PART IV, SUPP. 2,  
DATED: APRIL 27, 1954 SEC. 4  
FILE NO: 400-22,516





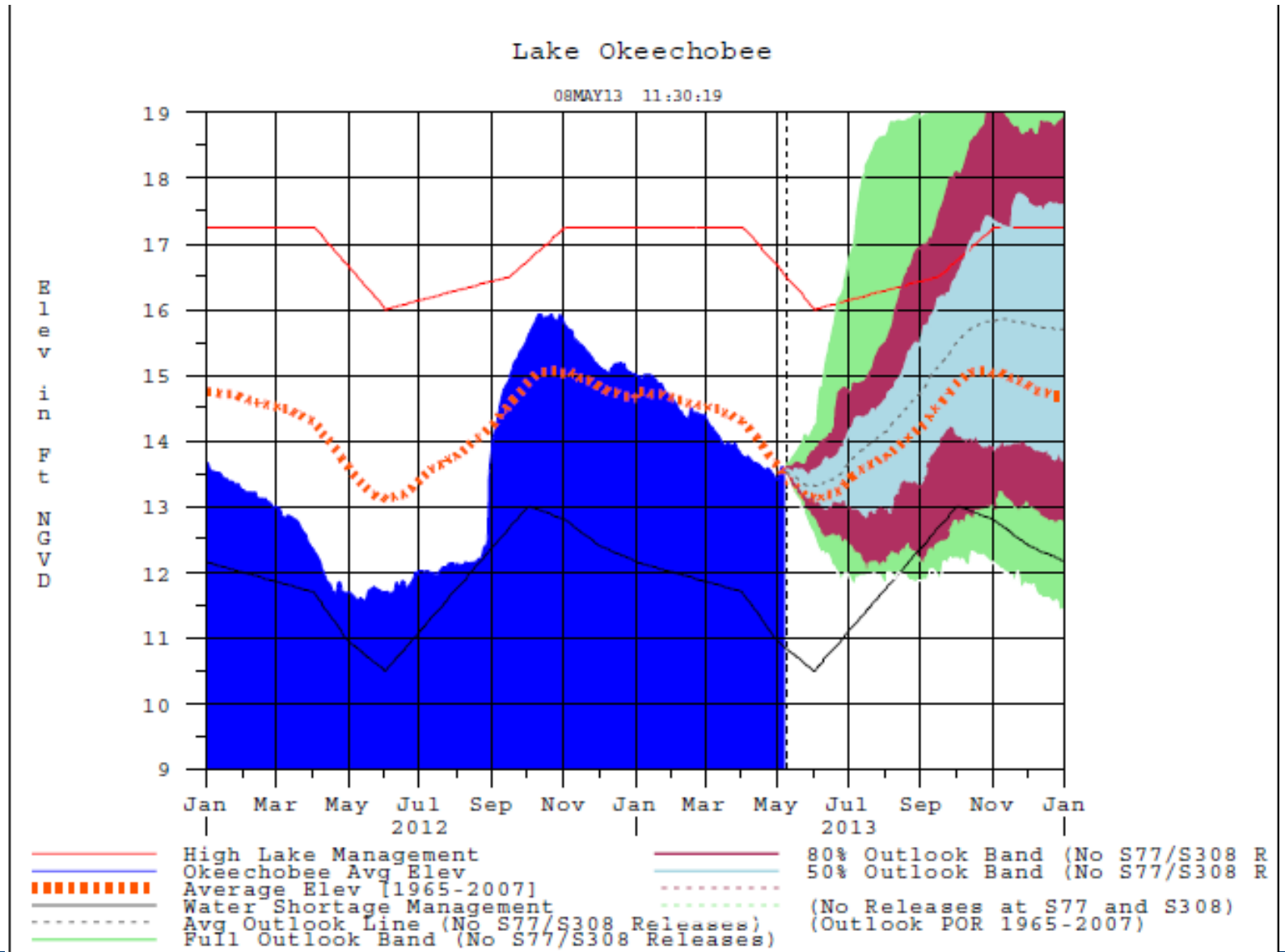
# Hurricane Katrina Alters Risk Management Approach



*Hurricane Katrina strikes land  
Aug 29, 2005*

- World-wide impact, re-evaluation of projects
- Corps changes procedures for managing dams and levees
- Interagency Task Force Report recommends *robustness, resiliency and redundancy* for all dams

# CURRENT LAKE LEVEL

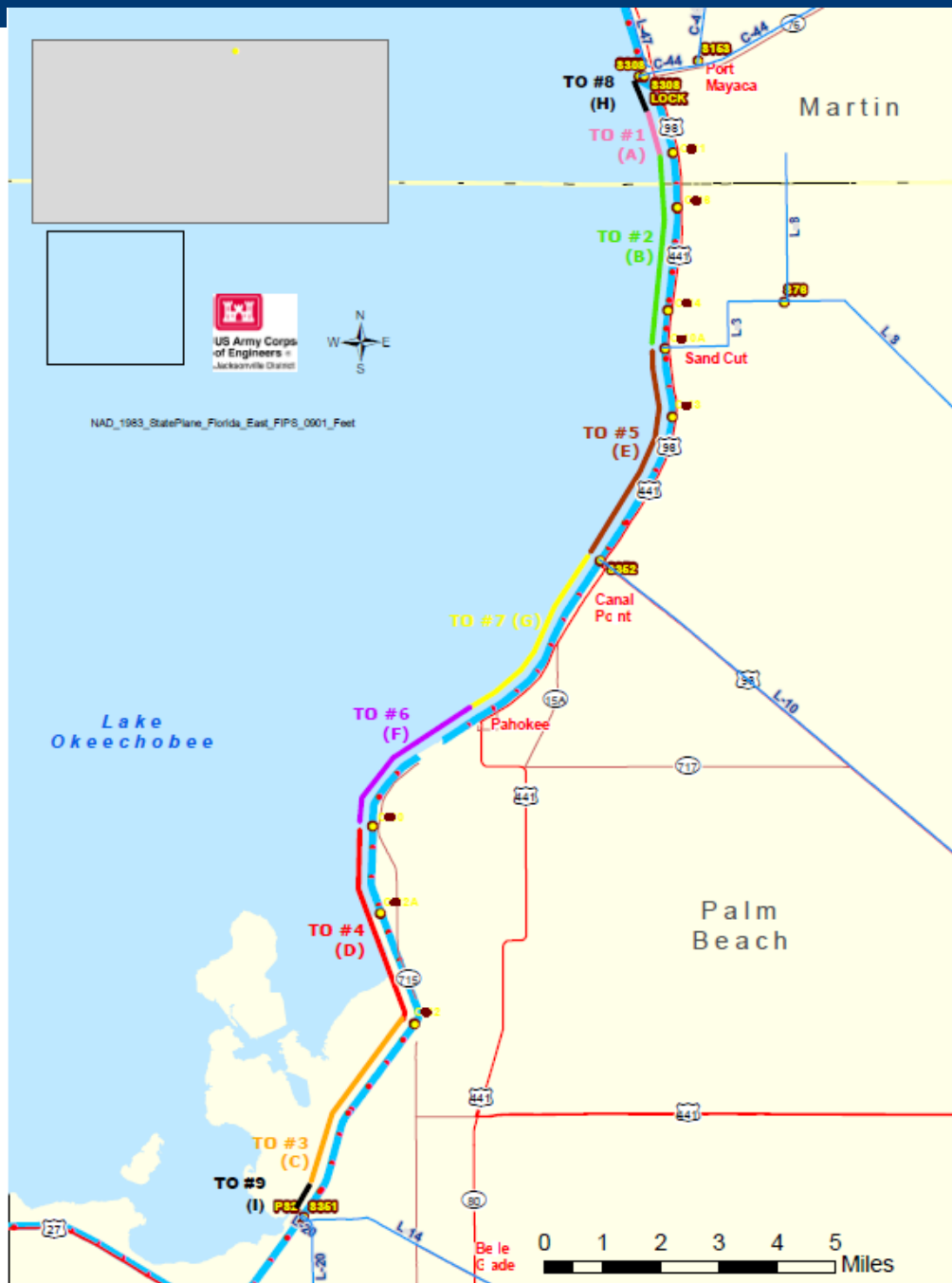


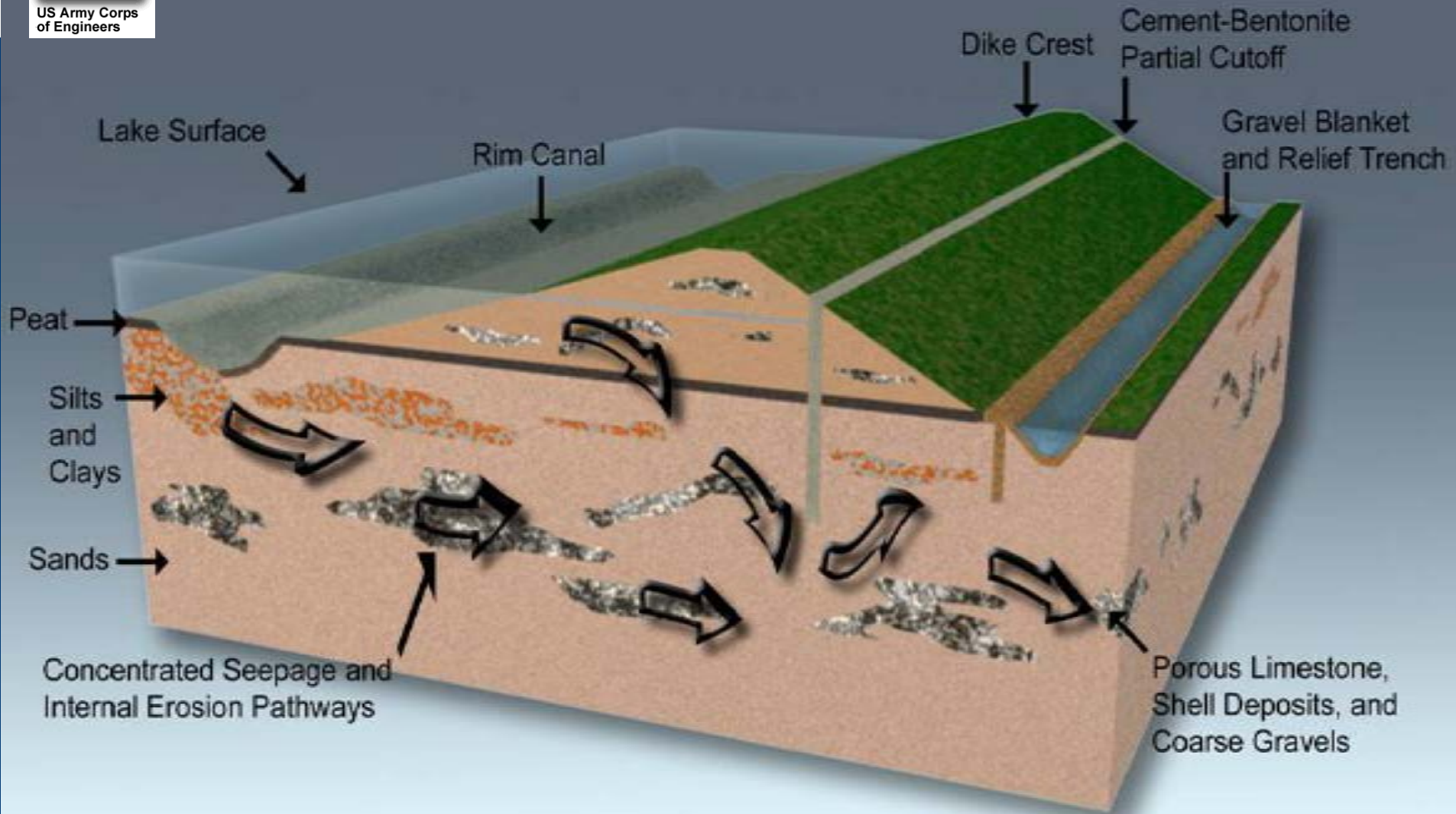


# Herber Hoover Dike Rehabilitation Project



US Army Corps  
of Engineers





## Proposed Repair, Herbert Hoover Dike

# *Installation of Deep Cutoff Wall Using the Cutter Soil Mixing Method*



# Cutoff Wall Considerations

Cutoff Wall Cost Comparison			
Cutoff Wall Type	Average Cost (\$/square foot)	Number of Capable Technologies	Cost per mile (\$/mile)
Partial cutoff wall (80 feet)	\$24	7	\$10,000,000
Full depth cutoff wall (200 feet)	\$100 to \$250	2	\$106,000,000 to \$264,000,000

- A partial cutoff wall is more cost effective in reducing seeps and pipes
- Both walls require landside features for relief of uplift pressure
- A fully penetrating cutoff wall will stop seepage and impact the regional groundwater supply; additional features needed to provide water supply



# *PERFORMANCE CRITERIA*

- Continuous CB Wall
- Panel Width < 34 in (25 in)
- Avg. CSM Panel Depth 70 ft
- UC Strength 100 – 500 psi
- $K = 1 \times 10^{-6}$  cm/sec or less







# Pre-Drilling Activities



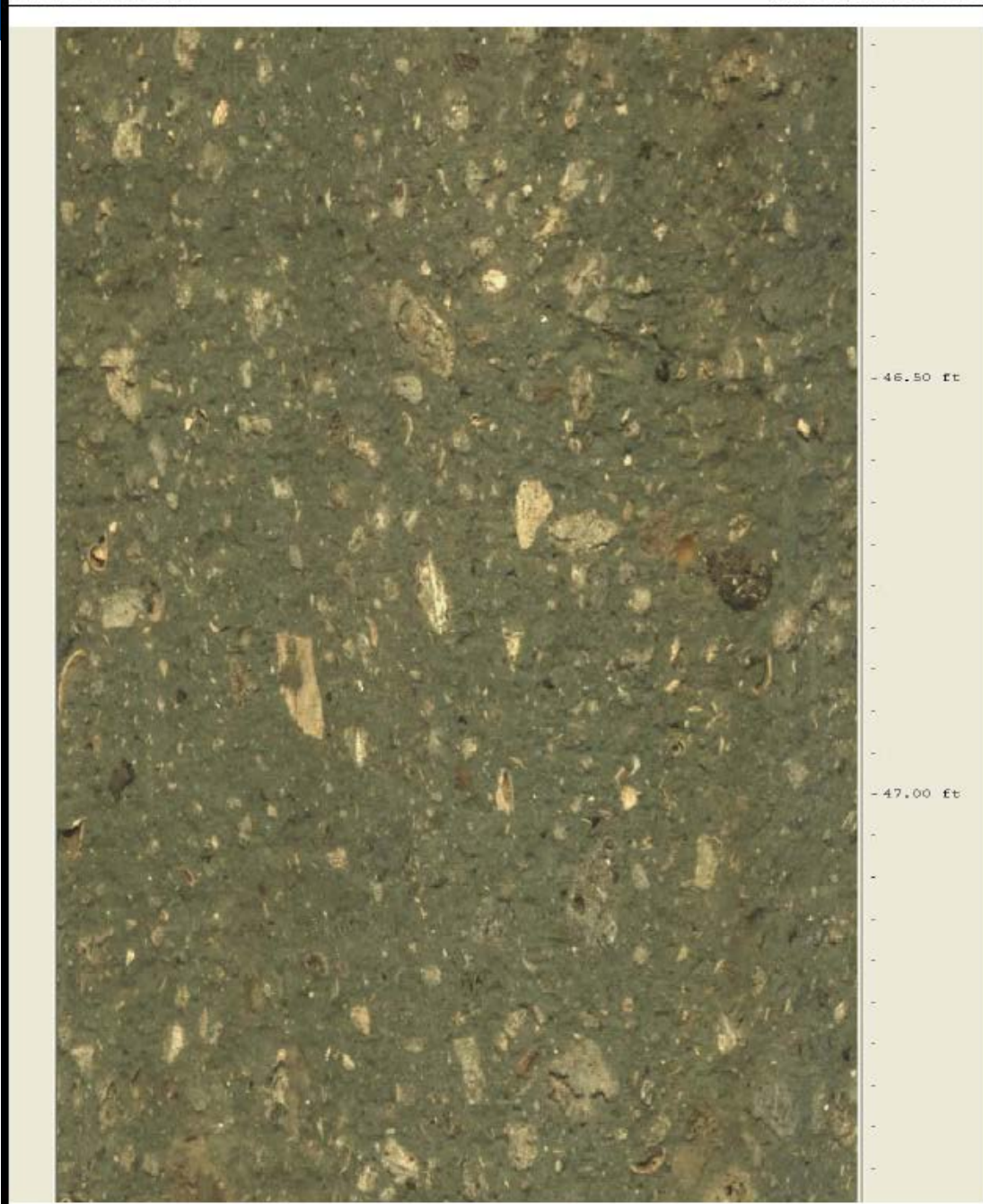
# Cutter Soil Mixer (CSM)



JAN 23 2009

## Cutter Soil Mixing



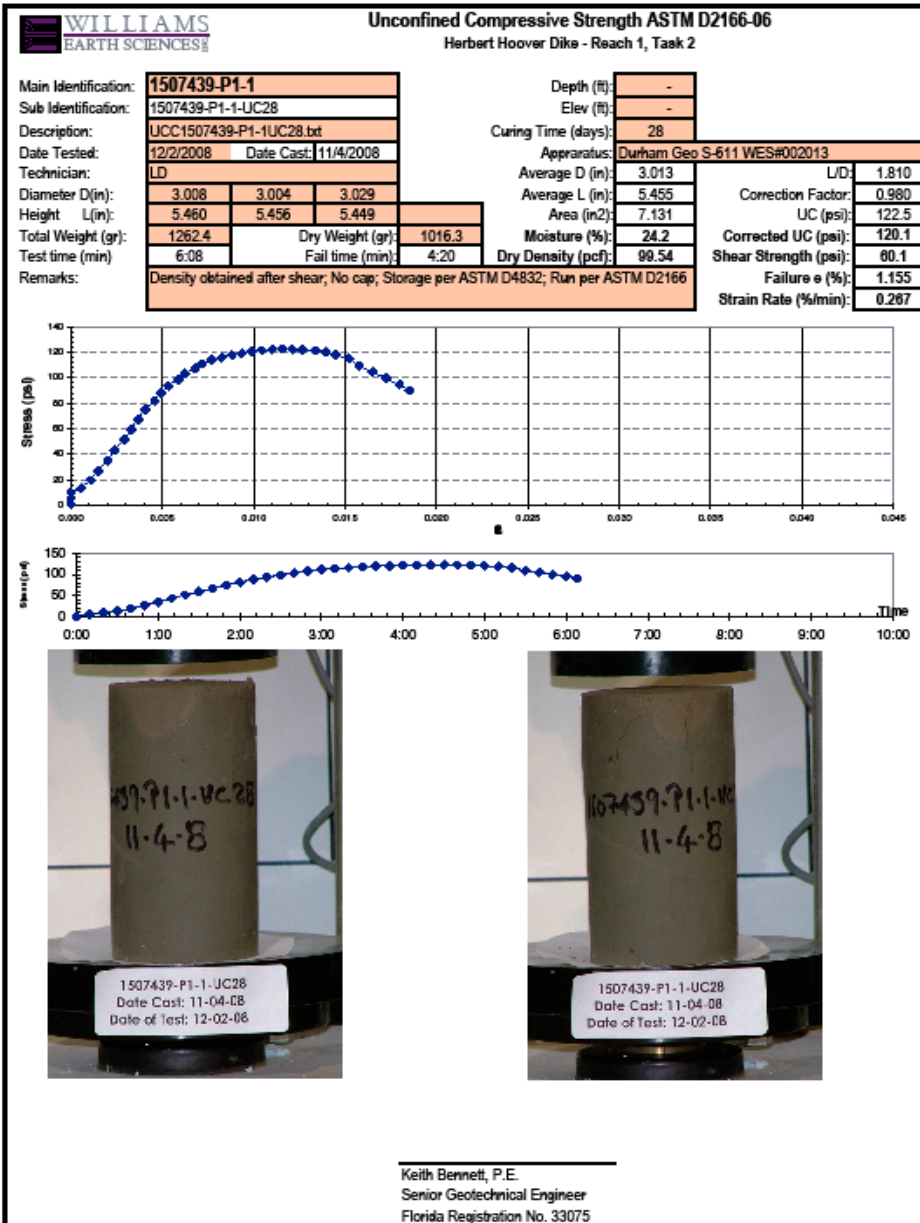


DEPTH: from 46.08 ft to 47.49 ft

# ***Construction Quality Control Procedures***




- Daily CQC reporting
- Bulk Sampling
- Post Placement Sampling
- Verification Core Drilling
- Inclinator Testing
- Video Logging
- Falling Head Permeability Field Tests
- Unconfined Compressive Strength Tests





# ❖ Unconfined Compressive Strength

# ❖ ASTM D2166

WILLIAMS EARTH SCIENCES, INC.		VERIFICATION BORING LOG		DIVISION Jacksonville		INSTALLATION		SHEET 1 OF 3 SHEETS	
PROJECT Herbert Hoover Dike Demonstration section 1504+50.6				SIZE & TYPE OF BIT Diamond Stepbit Bottom Dish					
BORING DESIGNATION 1504506				TYPE OF DRILL RIG CME 650					
LOCATION COORDINATES X= -1.9 Y= -0.3				TOTAL NUMBER CORE BOXES 5					
DRILLING AGENCY Williams Earth Sciences				ELEVATION GROUND WATER 14.33 (assumed based on reported lake level)					
CONTRACTOR FILE NO.				DATE BORING START 11/18/06 COMPLETED 11/18/06					
DEPTH OF BORING (ft) 55				ELEVATION TOP OF BORING 35.14					
NAME OF DRILLER Jim Spoon				MAX SAMPLE LENGTH (in) 60					
				CORE SAMPLE DIAMETER (in) 3.3					
				BOREHOLE LOGGER: Gabrielle Enos					
ELEV. DEPTH	MATERIAL DESCRIPTIONS & NOTES	BOX #	GEAR PRESS. (psi)	RPM	TIME (min:sec)	REC (ft)	ROD (ft)	Q <sub>u</sub> (psi)	PICTURE
35.1 0	No samples collected in upper 5 feet. Top of wall begins at 5 ft bls								
30.1 5	Dark greenish-grey cementitious soil with shell inclusions	2nd	200	60	0.2	100	95		
		2nd	200	60	4:30				
		1 2nd	200	60	4:42				
		2nd	200	60	4:34				
		2nd	200	60	4:58				
25.1 10	Dark greenish-grey cementitious soil with shell inclusions  hard drilling at 3 feet into run	2nd	200	60	8:24	100	100		
		2nd	200	60	9:24				
		1 2nd	200	60	13:19				
		2nd	200	60	5:17				
		2nd	200	60	6:15				
20.1 15	Dark greenish-grey cementitious soil with shell inclusions  UCC Sample at 17-18 ft bls	2nd	200	60	3:54	100	767.1 844.6		
		2nd	200	60	4:22				
		2 2nd	200	60	4:01				
		2nd	200	60	4:33				
		2nd	200	60	5:04				
15.1 20									

# ❖ Verification Coring & Photo Log





2008 11 24

## REPORT OF FALLING HEAD PERMEABILITY TEST

Project Name: HHD-Reach 1 Contractor: Bauer  
 Hole Number: 1504506 Borehole Logger: Williams  
 Station: 1504+50.6 Driller: Williams  
 Date: 11/21/2008

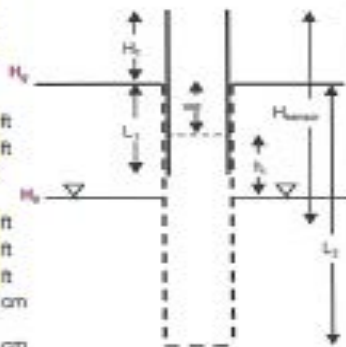
Elevation of Ground Surface in NAVD88 ( $H_g$ ): 35.14 ft  
 Elevation of Groundwater ( $H_w$ ): 14.41 ft  
 (Assume groundwater level equals water level of Lake)

Height of Standpipe Above Ground ( $H_s$ ): 0.35 ft  
 Depth to Bottom of Packer ( $L_1$ ): 0 ft  
 Depth to Bottom of Tested Zone ( $L_2$ ): 56.65 ft  
 $L = (L_2 - L_1) \times 30.48 = \underline{1726.652}$  cm

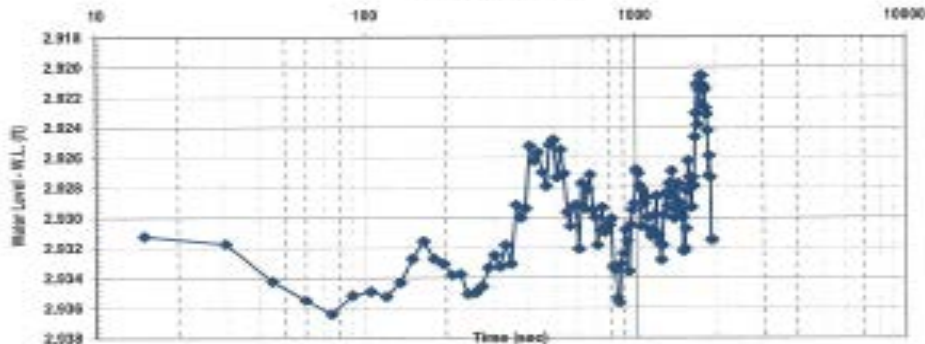
Borehole Diameter ( $D$ ): 4.8 in.  $\times 2.54 = \underline{12.192}$  cm  
 Standpipe Diameter ( $d_s$ ): 4.8 in.  $\times 2.54 = \underline{12.192}$  cm

Test Start Time: 10:11:30 End Time: 10:49:30  
 Total Test Time (minutes): 32.25  $k_p = \underline{4.4E-10}$  cm/s (average)  
 $k_p = \underline{4.4E-10}$  cm/s (between begin and end times)

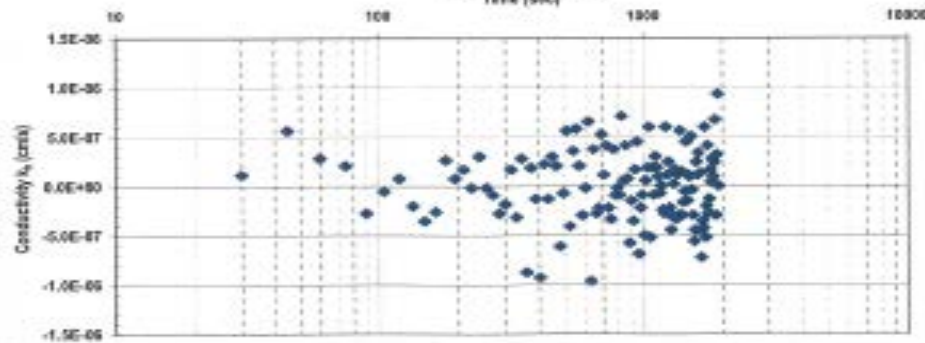
Note: Measurement of water levels (W.L.), elevations, and depths shall be accurate to increments of 0.1 feet, diameters to 0.1 inches, or less



Falling Head Test



Falling Head Test



❖ Open Hole  
 Falling Head  
 Permeability  
 Tests

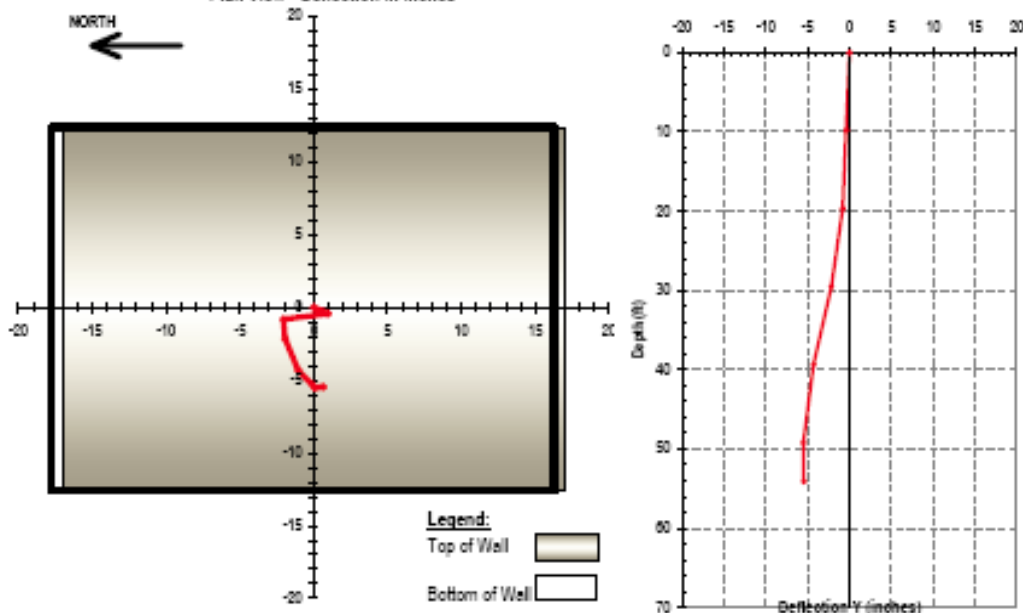
❖ Vibrating Wire  
 Piezometer



2008 11 21

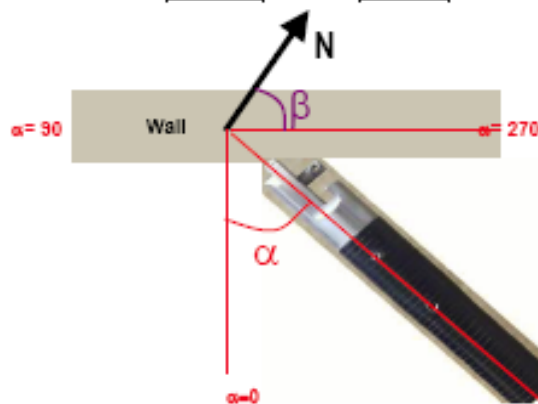
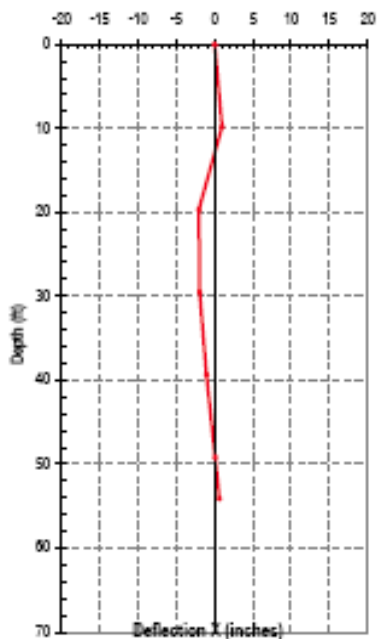
# Inclinometer Graphic Results

Plan View - Deflection in inches



Project: Herbert Hoover Dike - Reach 1, Task 2  
 Boring: Panel 1504506, uphole readings  
 Source: rd Settings\swes\My Documents\P1504506uphole.ASC  
 Inspector: -

Wall End Deviation: -0.750 inches along wall length  
 -0.120 inches along wall width  
 $\alpha$ : 90 degrees  Built-in Boretrack Data  
 Wall compass reading  $\beta$ : 180 degrees (From 0 to 360)  
 Hole Depth: 55.0 ft  
 Average Deflection: 0.9 degrees X: 0.08 inches along length  
 Max Deflection: 5.51 inches Y: -5.51 inches along width

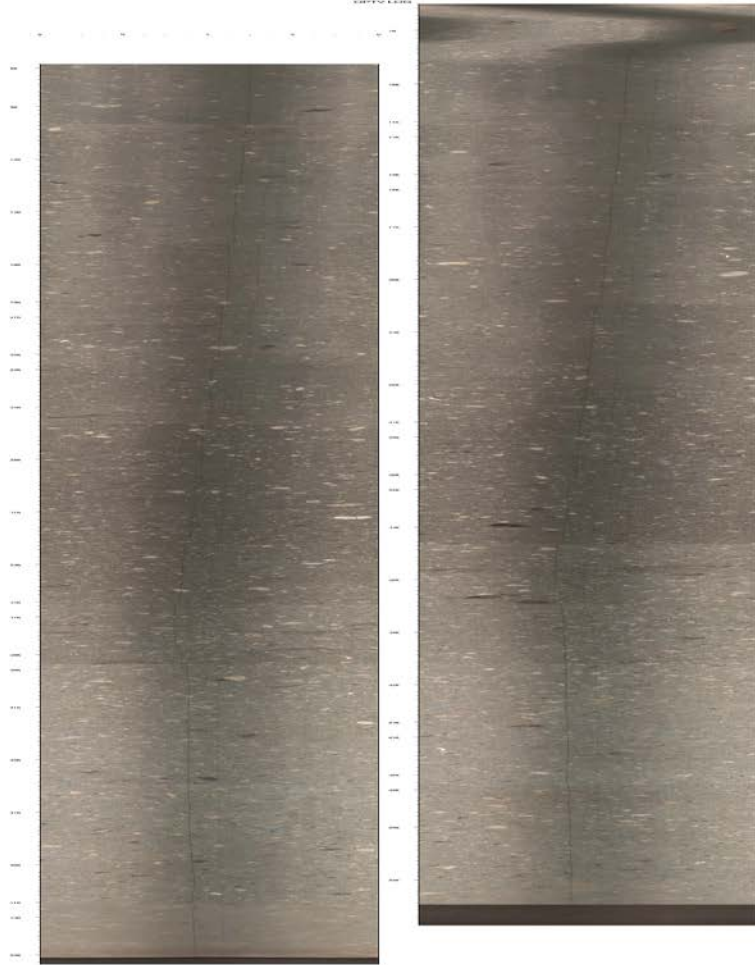


## ❖ Inclinometer Measurement





PROJECT INFORMATION	
PROJECT NO.	
DATE	
LOCATION	
CLIENT INFORMATION	
CLIENT NAME	
CONTACT PERSON	
PHONE	
EMAIL	
LOG INFORMATION	
LOG NO.	
DATE	
TIME	
DEPTH	
LOG DESCRIPTION	
LOG TYPE	
LOG STATUS	
LOG COMMENTS	

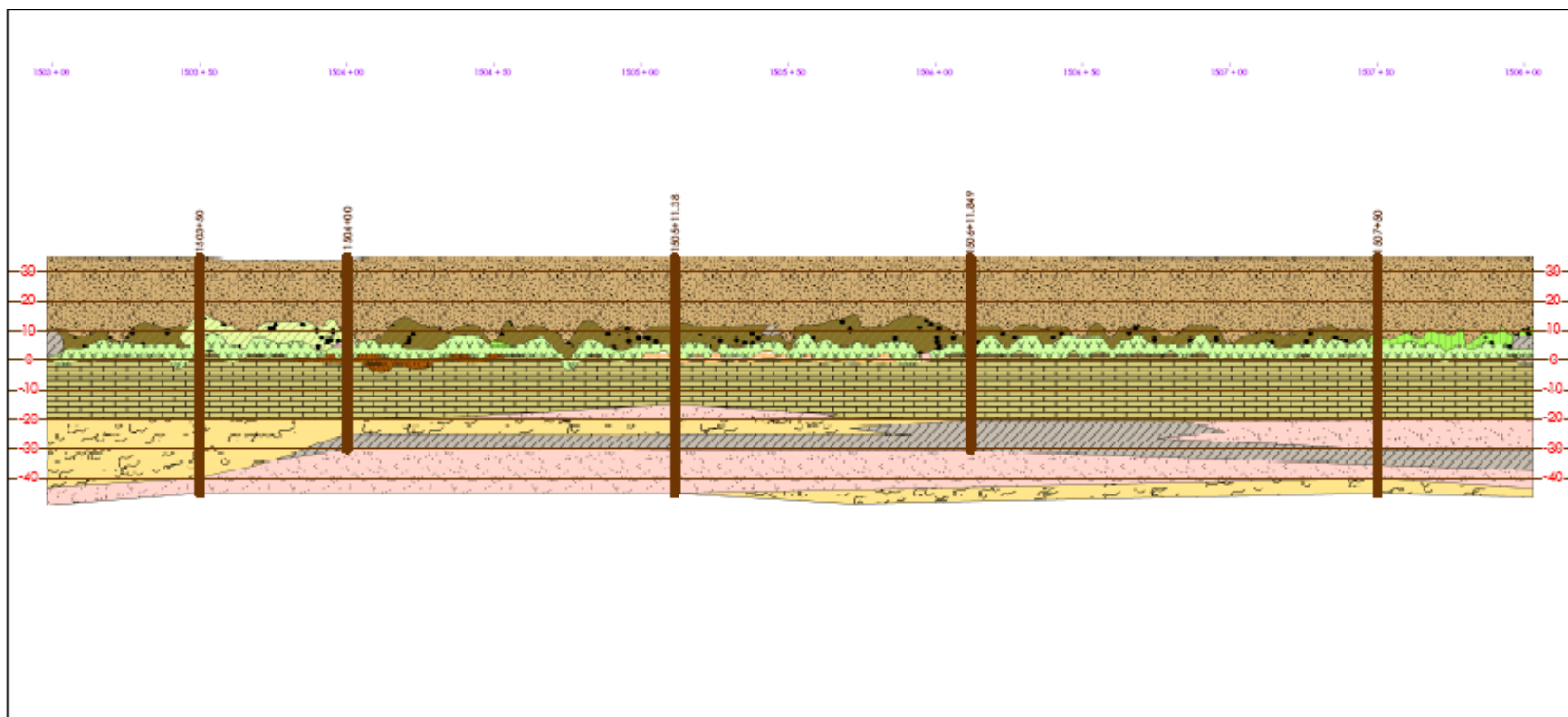


- ❖ Downhole Video Log
- ❖ 360 degree view

# *DATA MANAGEMENT*

- Collection and transmission of data
- Remote sites with complex instrumentation and onerous construction QC requirements
- Automated Data Management Systems
- GIS integration
- Web Based Information Management System





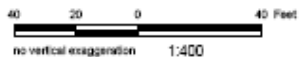
**LEGEND**

1503+00 Station Number  
 30 Elevation (feet NAVD88)

1503+00  
 Rotasonic Boring  
 (not to scale horizontally)

**Lithology**

- Sand (Fill Material)
- Sandy Clay
- Sand with Limestone
- Interbedded Silt, Silty Sand and Clay
- Silt and Silty Sand
- Sand
- Clay
- Peat
- Shell Hash with Sand
- Interbedded Limestone and Silty Sand
- Limestone



**DEPARTMENT OF THE ARMY**  
 JACKSONVILLE DISTRICT, CORPS OF ENGINEERS  
 JACKSONVILLE, FLORIDA

NAVD88 - North American Vertical Datum of 1988

Herbert Hoover Dike  
 Rehabilitation Reach 1  
 Seepage Cutoff Wall, Task Order No. 2  
 Martin and Palm Beach Counties, Florida  
 Contract No. W912E9-07-D-0011

Profiles - Demonstration Section  
 Station 1503+00 to 1508+00  
 Existing Lithology

Rev. 0: 25 FEB 2009

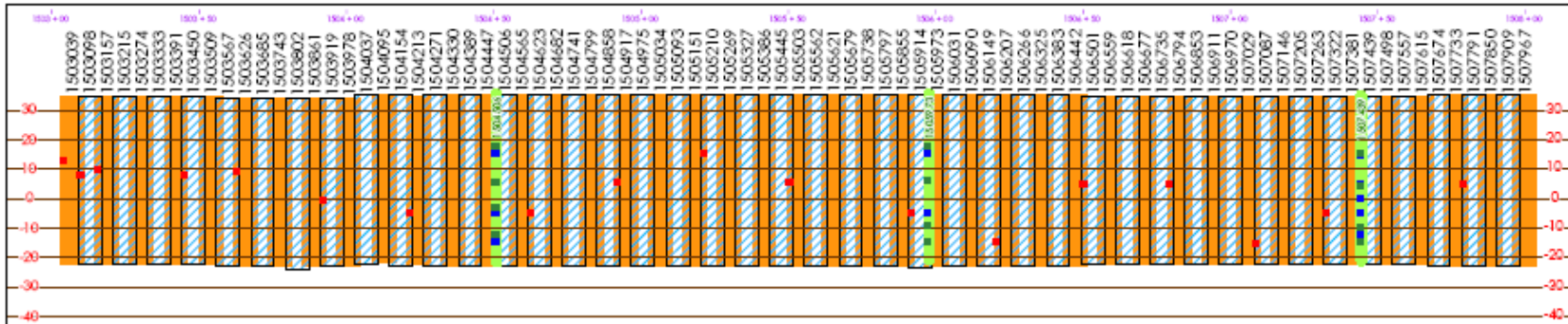
Rev. 1: 05 MAR 2009

Drawn By:  
 J.R. Drawing No.

Ckd By:  
 D.S. AB-01  
 Sheet 1 of 4



Bauer Foundation Corporation  
 13020 Byrd Legg Drive, Odessa, FL 33556  
 727.536.4748



**Laboratory Results:**

Sample ID	Panel Type	Sample Date	Type of Sample	Sampling Depth (ft)	UCS 7-day (psi)	UCS 14-day (psi)	UCS 28-day (psi)	Perns 7-day (ksi)	Perns 14-day (ksi)	Perns 28-day (ksi)
Demonstration Section Station 1503+00 to 1508+00 - Daily Bulk Sample Summary										
150339-C2	P	10/16/00	Daily Bulk Sample	22.0	Soft	23.3	194.4	Soft	Soft	3.97E-04
150357-C2	P	10/17/00	Daily Bulk Sample	25.0	Soft	24.4	21.4	Soft	Soft	1.40E-01
150396-C1	S	10/18/00	Daily Bulk Sample	27.0	Soft	191.1	426.3	Soft	3.20E-04	1.50E-04
150409-C2	S	10/20/00	Daily Bulk Sample	27.0	Soft	43.3	241.1	Soft	Soft	1.25E-02
150339-C1	P	10/21/00	Daily Bulk Sample	25.0	Soft	47.8	286.3	Soft	2.08E-01	3.55E-04
150319-C2	S	10/22/00	Daily Bulk Sample	25.0	Soft	22.4	173.4	Soft	Soft	7.30E-03
150421-C1	P	10/23/00	Daily Bulk Sample	48.0	Soft	24.2	Soft	Soft	Soft	6.90E-03
150435-C2	S	10/24/00	Daily Bulk Sample	48.0	Soft	21.8	Soft	Soft	Soft	1.76E-04
150481-C1	P	10/25/00	Daily Bulk Sample	30.0	Soft	22.5	Soft	Soft	Soft	1.91E-02
150216-C2	S	10/27/00	Daily Bulk Sample	26.0	Soft	21.4	Soft	Soft	Soft	3.22E-03
150500-C1	P	10/28/00	Daily Bulk Sample	36.0	Soft	24.8	21.4	Soft	Soft	6.90E-03
150414-C2	S	10/28/00	Daily Bulk Sample	48.0	Soft	24.8	11.4	Soft	Soft	2.92E-04
150507-C1	P	10/29/00	Daily Bulk Sample	30.0	Soft	22.3	Soft	Soft	Soft	6.90E-03
150601-C2	S	10/31/00	Daily Bulk Sample	36.0	Soft	24.8	18.4	Soft	Soft	6.90E-03
150679-C1	P	11/01/00	Daily Bulk Sample	36.0	Soft	22.8	Soft	Soft	Soft	1.19E-02
150701-C2	S	11/02/00	Daily Bulk Sample	36.0	Soft	22.8	18.1	Soft	Soft	4.26E-03
150732-C1	P	11/04/00	Daily Bulk Sample	48.0	Soft	43.8	211.4	Soft	2.08E-01	2.72E-03
150739-C2	S	11/05/00	Daily Bulk Sample	36.0	Soft	24.8	73.5	Soft	Soft	2.77E-03

Sample ID	Panel Type	Sample Date	Type of Sample	Sampling Depth (ft)	UCS 28-day (psi) (see Note 1)	Perns 28-day (ksi)
Demonstration Section Station 1508+00 to 1509+00 - Post-Placement Sample Summary						
150406-P1	S	10/24/00	Post-Placement	20	232.1 / 241.4	2.485-06
150406-P2	S	10/24/00	Post-Placement	40	139.3 / 196.4	9.44E-06
150406-P3	S	10/24/00	Post-Placement	60	46.3 / 115.2	2.37E-06
150507-P1	P	10/29/00	Post-Placement	20	19.5 / 13.5	2.93E-07
150507-P2	P	10/29/00	Post-Placement	40	25.8 / 26.9	1.90E-11
150507-P3	P	10/29/00	Post-Placement	60	7.5 / 7.3	1.30E-10
150734-P1	S	11/04/00	Post-Placement	20	120.1 / 125.6	3.22E-06
150734-P2	S	11/04/00	Post-Placement	40	60.1 / 83.4	3.15E-06
150734-P3	S	11/04/00	Post-Placement	60	30.4 / 46.7	8.4E-08
150734-P1A	S	11/06/00	Post-Placement	20	35.1 / 316.8	1.244E-01
150734-P2A	S	11/06/00	Post-Placement	35	38.5 / 375.6	1.163E-01
150734-P3A	S	11/06/00	Post-Placement	47	54.7 / 833.5	1.376E-01

Note: 1 - UCS 16-day  
1. Values shown represent test results from two separate cylinders obtained from the same sample batch and tested individually.

Abbreviations: UCS = Unconfined Compressive Strength  
Pern = Permeability Test

Sample ID	Panel Type	Sample Date	Type of Sample	Yielded	Sampling Depth (ft)	% Recovery	UCS 28-day (psi)	Pern 28-day (ksi)
Demonstration Section Station 1503+00 to 1508+00 - Verification Borehole Sample Summary								
150405-V01	S	11/02/00	Verification Core	Yes	17-19		642.7	Pass/Fail
150405-V02	S	11/02/00	Verification Core	Yes	29-31	88.6	343.5	Pass/Fail
150405-V03	S	11/02/00	Verification Core	Yes	37.9-39.3		72.9	4.4E-10
150405-V04	S	11/02/00	Verification Core	Yes	47-49		67.5	Pass/Fail
150607-V01	P	11/04/00	Verification Core	Yes	15.5-18		811.8	Pass/Fail
150607-V02	P	11/04/00	Verification Core	Yes	24-25	100.0	511.4	Pass/Fail
150607-V03	P	11/04/00	Verification Core	Yes	43.6-45		683.4	4.2E-07
150734-V01	S	12/01/00	Verification Core	Yes	29-31		88.6	Pass/Fail
150734-V02	S	12/01/00	Verification Core	Yes	39-39		225.8	Pass/Fail
150734-V03	S	12/01/00	Verification Core	Yes	39-37.1	88.7	875.7	6.1E-08
150734-V04	S	12/01/00	Verification Core	Yes	44-45		828.3	Pass/Fail

**LEGEND**

1503+00 Station Number

∞ Elevation (feet NAVD83)

**Panel Type**

Primary Panel

Secondary Panel

1503039 Panel Number/Station

**Sample Type**

Daily Bulk Sample

Post-Placement Sample

Verification Core Sample

Verification Borehole (not to scale horizontally)

Notes:  
Mix Design #1 used between stations 1503+00 and 1508+00

NAVD83 - North American Vertical Datum of 1983

40 20 0 40 Feet

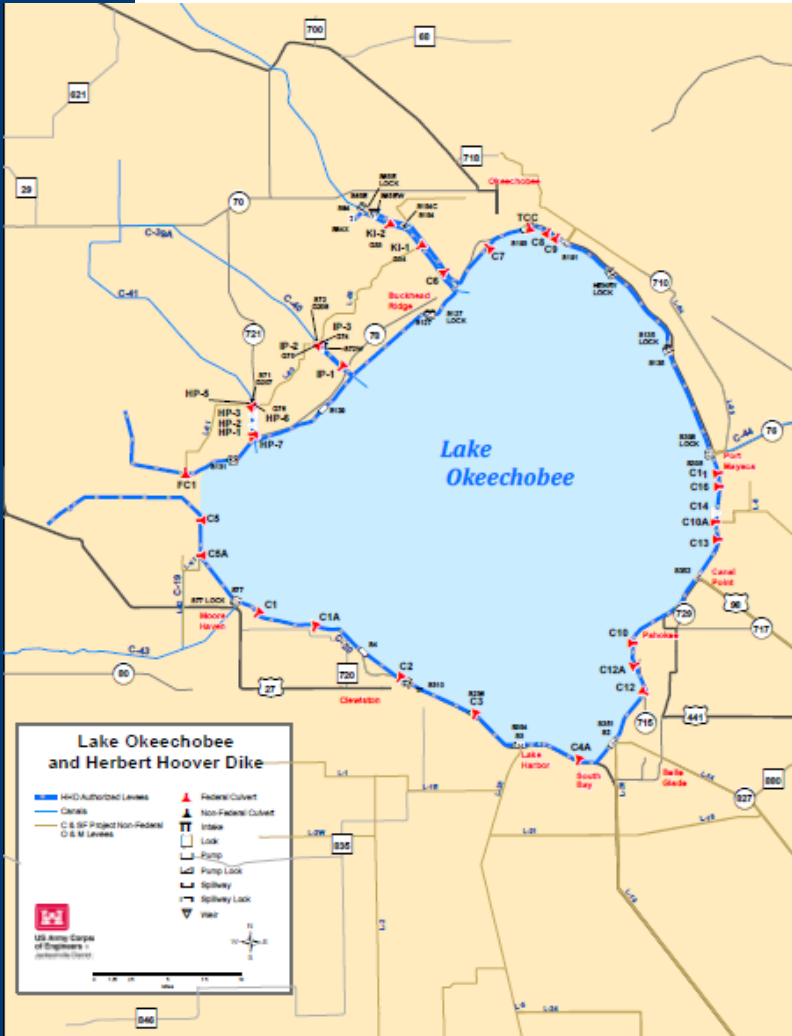
no vertical exaggeration 1:400

Herbert Hoover Dike Rehabilitation Reach 1 Seepage Cutoff Wall, Task Order No. 2 Martin and Palm Beach Counties, Florida Contract No. W912E9-07-D-0011		Rev. 0: 02 MAR 2009
		Rev. 1: 05 MAR 2009
Drawn By: J.R.	Drawing No.	
Ckd By: D.S.	AB-04	Sheet 4 of 4



Bauer Foundation Corporation  
13020 Byrd Legg Drive, Odessa, FL 33556  
727.536.4748

# CULVERTS



# QUESTIONS



***Thank you for your time and consideration  
today in discussing the Herbert Hoover Dike  
Rehabilitation Project.***



**Geosyntec**   
consultants

engineers | scientists | innovators